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# PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC

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**Disciplines:** All

**History:** Revised 12/05/22 under 2022 CBC  
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Division of the State Architect (DSA) documents referenced within this publication are available on the [DSA Forms](#) or [DSA Publications](#) webpages.

## PURPOSE

This Interpretation of Regulations (IR) clarifies requirements relating to pre-check (PC) submittals to promote uniform statewide criteria for code compliance in design and in plan review of relocatable buildings for projects under DSA jurisdiction. The PC Design Criteria documents were created by DSA as a means for the responsible engineer to demonstrate code compliance when developing and submitting construction documents for DSA review.

The provisions of this IR are intended to be a tool to identify and highlight the common and unique, critical and/or overlooked code requirements that must be considered and incorporated into the design, as applicable, to provide a complete and consistent set of construction documents accepted at all DSA regional offices. Other methods proposed by design professionals to solve a particular issue may be considered by DSA and reviewed for code and regulation compliance, subject to concurrence of DSA Codes and Standards Unit. For methods not specifically prescribed in the code, see California Building Code (CBC) Section 104.11.

Appendix A below is provided as a guide to assist design professionals and DSA plan reviewers when preparing and reviewing site-specific project applications that incorporate PC relocatable buildings designed in accordance with this IR.

## SCOPE

The provisions of this IR apply to 2022 PC plans for new relocatable buildings submitted to DSA under the 2022 CBC. As defined in the CBC, a relocatable building is any building with an integral floor structure which is capable of being readily moved. This document does not address permanent modular buildings, cargo container conversions, and modular elevator towers.

As noted in Bulletin (BU) 18-01: *Applicability of Pre-Check (PC) Design Criteria for Non-PC Projects*, these provisions shall also be considered and incorporated in site-specific submittals for structures of the same project type, even if the submittal is not part of a PC application.

## BACKGROUND

The PC approval process is intended to streamline DSA plan review by providing a procedure for approving the design of commonly used structures prior to the submittal of plans to DSA for construction projects. The PC approval process allows designers to incorporate designs for structures that have already been “pre-checked” by DSA into their plans for actual site-specific construction projects. The design criteria provided in this document are neither regulations nor law and are not appropriate for verbatim inclusion in project specifications. The design professional in charge is responsible for specifying and detailing requirements for each project.

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Additional information regarding the design and site application of PC structures and relocatable buildings can be found in the following documents:

- *IR 16-1: Design and Construction Requirements for Relocatable Buildings and Modular Elevator Towers.*
- *Policy (PL) 07-02: Over-the-Counter Review of Projects Using Pre-Check Approved Designs.*
- *Procedure (PR) 07-01: Pre-Check Approval.*

### 1. GENERAL

#### 1.1 Pre-Check Approval Requirements

See PR 07-01 for a more detailed list of items that are required for all PC submittals. The documents required to be submitted for PC approval are listed on form *DSA 3: Project Submittal Checklist*. Site-specific information is not necessary as that information will be provided when a specific construction project is submitted for DSA review. ←

#### 1.2 Cover Sheet and General Notes

**1.2.1** In accordance with PR 07-01 Section 2.4 the first sheet(s) of the PC drawings shall include a design information section that defines the basis of the PC design. Refer to PR 07-01 Appendices B and C and the remainder of this IR for required content of the design information section.

**1.2.2** The PC construction documents shall include complete and comprehensive general notes and/or specifications as required for construction and inspection. It is common for PC construction documents to consist of drawings only without a book specification or project manual. Refer to PR 07-02 Appendix B, Footnote 6. In this case, the PC drawings shall include information that might otherwise be communicated in a project manual or book specification. For each primary material or group of the materials, the following information shall be specified in the construction documents when applicable:

**1.2.2.1** Required material properties, including compliance with American Society for Testing and Materials (ASTM) specifications when applicable.

**1.2.2.2** Proprietary products name, manufacturer, and evaluation report number. Refer to Section 1.14 below.

**1.2.2.3** Quality control performed by the supplier.

**1.2.2.4** Standards for the execution of the work, including associated tolerances. References to recognized standards are acceptable.

**1.2.2.5** Required qualifications of personnel performing the work for each applicable trade.

**1.2.2.6** Product and material finish where required for weather protection or safety.

**1.2.2.7** Quality assurance tests and frequency requirements, including citation of ASTM standards when applicable, not covered by Section 1.3 below.

#### 1.3 Structural Tests and Special Inspections

Provide example form(s) *DSA 103: List of Required Structural Tests and Special Inspections* on the drawings. See PR 07-01 Section 2.5 for additional information.

**1.3.1** Example form DSA 103 will be used as a guide to develop a site-specific form DSA 103 for the site-specific project. Example form(s) on the PC drawings will be crossed out when the site-specific form DSA 103 is provided during plan review.

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**1.3.2** The example form DSA 103 will include both in-plant and on-site testing and inspection requirements. Manufacturers shall be involved in the coordination of in-plant testing and inspection with the project inspector and Laboratory of Record (LOR) of the site-specific project application using the PC design prior to commencing fabrication.

**1.3.3** Only the site-specific form DSA 103 can identify exemptions from the required structural tests and special inspections; therefore, the Appendix of the example form(s) DSA 103 shall not be included on the PC drawings. Applicability of exemptions may be considered during plan review for site-specific applications, shall be justified by the project design professional, and is subject to DSA review and approval. Refer to Appendix A below for additional information

**1.3.4** See Section 2.10 below for specific structural tests and special inspection requirements that often occur in Relocatable Buildings.

#### **1.4 Options and Variations**

Provide checkboxes of options and variations if there is more than one configuration or design loading criteria. For a PC that includes any restrooms, the plans, elevations, and details shall be presented separately on stand-alone sheets for each age-related user group (e.g., age 3-4, age 5-8, etc.). See PR 07-01 Section 3 for more details, including the maximum number of options permitted in a single PC

#### **1.5 Relocation Option**

If the PC allows for relocation of the building to another site, provide drawings within the PC that are specifically for submittal of the relocation. These drawings shall include the foundation plan, foundation details, superstructure to foundation connection details, and module interconnection details.

#### **1.6 Design Parameters**

The PC drawings shall provide on the coversheet (and subsequent sheets as necessary) Design Information as defined in PR 07-01 Section 2.4 and Appendix B. In accordance with Section 8 of this IR, Photovoltaic (PV) requirements shall also be included in the Design Information section. If the PC includes design variations for multiple tiers or levels of the same design parameter(s), all or part of the Design Information should be presented in a checklist format and provide general direction to future users (design professionals and plan reviewers) for the application of the PC to site-specific projects. The checklist shall include any cut sheets of boards, boxes and equipment to be mounted on the structure, including weights and dimensions. Additionally, refer to and coordinate with PL 07-02 Section 3, which summarizes common site-specific parameters to be verified at Over-the-Counter (OTC) plan reviews.

#### **1.7 Required Photovoltaic (PV) Systems**

When a PV system is required by the California Energy Code for a PC building configuration, the PV system design shall be in accordance with *IR 16-8: Solar Photovoltaic and Thermal Systems Review and Approval Requirements*. The PC design shall show that the structure can support the required system loads when the building is supporting the PV system. The PV system design may be included in the PC or may be submitted with the PC at the time of the siting application.

#### **1.8 Risk Category and Occupant Load**

PC drawings shall indicate the maximum Risk Category (RC) the structure is designed for in the Design Information section of the coversheet. In addition, a Code Analysis shall be shown on the coversheet to indicate the intended Use and Occupancy. The DSA reviewer of the site application shall verify the RC of the PC structure as it applies to the site in accordance with CBC Section 1604A.5. The Code Analysis shall include line items to indicate selection of Use

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and Occupancy classification per CBC Chapter 3, Occupant Load Factor (OLF) per CBC Table 1004.5, and determination of RC per CBC Table 1604A.5, to be completed by the Design Professional at time of OTC or project submittal. Refer to Appendix A below for additional information.

**1.9 Flood Zone**

The PC design shall comply with CBC Section 1612A and *PR 14-01: Flood Design and Project Submittal Requirements*.

**1.9.1** The design information section shall include a note requiring the building pad be raised above the design flood elevation. See PL 07-02 Section 4.9 for additional information.

**1.9.2** The design information section shall include a note stating that when the site-specific project is located in a flood zone other than Zone X, a letter from a geotechnical engineer (bearing his/her stamp and signature) is required to validate the applicability of the allowable soil values listed on the PC drawings.

This note may include an exemption for the validation letter for projects located in Zone D (undefined) if a geotechnical report written for improvements on the same campus and in accordance with the current CBC is provided that either (1) confirms the site is not in a flood hazard zone or (2) acknowledges the flood hazard but confirms it does not result in a reduction of soil capacity values.

**1.9.3** The location of electrical elements shall conform to American Society of Civil Engineers Standard 24: Flood Resistant Design and Construction (ASCE 24) Section 7.2.

**1.10 Geohazard Reports**

The Design Information section shall indicate whether a geohazard report is required for the project. A geohazard report is required to be provided and approved by California Geological Survey (CGS) for all projects unless it meets the exemptions as outlined below, *and* the design does not rely on parameters which must be identified in a geohazard report such as site class and exposure categories.

**1.10.1 Existing Sites Outside of a Mapped Geologic Hazard Zone**

Single-story relocatable buildings 4,000 Square Feet (Sq. Ft.) or less complying with the requirements of *IR A-4: Geohazard Report Requirements*, Section 3.2.1 are exempt from the requirement to provide a geohazard report. The structures may be split into multiple seismically separated structures to stay below the 4,000 Sq. Ft. trigger.

**1.10.2 Sites within a Mapped Geologic Hazard Zone**

For single-story relocatable buildings 2,160 Sq. Ft. or less on non-permanent foundations and complying with the requirements of IR A-4 Section 2.6, DSA may waive the requirement for submittal and approval of a geohazard report by CGS if a geohazard report is provided that indicates there are no geologic hazards at the site.

**1.11 Weather Protection**

The PC design shall comply with the requirements of IR 16-1 Section 2.1. Cold-formed steel and structural steel members shall be protected by rust inhibitive coating where exposed to weather or moisture. Refer to CBC Section 2203A.1 and American Iron and Steel Institute (AISI) S240 Table A4-1.

**1.12 Sheet Index**

The PC drawings shall include a sheet index. When a PC includes multiple major options such that not all sheets are applicable to a given site-specific project application based on the option

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being used, the sheet index shall include check boxes. When the PC drawings are incorporated into a site-specific application, the submitted sheets will be identified by marking the check boxes (i.e., it is not necessary to strike out sheets that are not applicable). See PR 07-01 Appendix E for additional information.

**1.13 Stamps**

The PC drawings shall include the following:

**1.13.1** 2022 CBC PC Stamp per PR 07-01 Section 6.1.

**1.13.2** Two blank areas on each PC sheet title block as indicated in *PR 18-04: Electronic Plan Review for Design Professionals of Record*, Section 1: one for the PC Identification Stamp and one for the future site-specific Identification Stamp.

**1.14 Structural Products Acceptance**

All structural products shall meet the requirements set forth in *IR A-5: Acceptance of Products, Materials, and Evaluation Reports*. Code-based engineering calculations to support a manufactured product will be considered.

**2. RELOCATABLE BUILDING PROVISIONS****2.1 Building Configuration & Structural System**

Refer to CBC Sections 2211A, 2212A, or Chapter 23, as applicable.

**2.1.1** When the lateral system is light modular steel moment frames, the design, fabrication, and erection shall comply with CBC Section 2212A.1.2, as well as the American Institute of Steel Construction (AISC) 360 for structural steel or the American Iron and Steel Institute (AISI) S100 for cold-formed steel (CFS). Light modular steel moment frames designed per CBC Section 2212A.1.2 do not require compliance with AISC 341; however, special inspection and nondestructive testing shall conform to AISC 341 Chapter J per CBC Section 1705A.2.1.

**2.1.2** When the lateral system is light-framed shear walls, comply with CBC Chapter 23 or Section 2211A, as applicable.

**2.1.3** For CFS wall systems using flat strap bracing, comply with AISI S400 Section E3.

**2.1.4** For sheet steel sheathing systems, comply with AISI S240 Section B5.2.2.3.2 and AISI S400 Section E2.

**2.2 Protection Against Deterioration**

**2.2.1** Refer to IR 16-1 Section 2.1 for measures to protect the building from deterioration due to decay, termite damage and rust.

**2.2.2** Drainage shall be provided to prevent water from ponding beneath buildings. If the building is placed on raised wood foundations, such as wood sleepers placed on ground or asphalt that is planar (i.e., not lowered below adjacent grade), sloped drainage is not required away from the building if there is adequate slope at the site to prevent water from ponding beneath and immediately adjacent to buildings. For other conditions, such as lowered basin under building or concrete foundations, sloped drainage shall be provided away from the building in accordance with CBC Section 1804A.4. Refer to Appendix A below for additional information.

**2.2.3** Regardless of construction material type or building size, the net free cross-ventilation area shall comply with CBC Section 1202.4 at under-floor spaces and CBC Section 1202.2 for enclosed spaces above ceilings.

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**2.2.4** Exterior balcony and elevated walking surfaces exposed to water shall comply with the requirements of CBC Sections 107.2.5 and 110.3.7.

**2.2.5** Wood structural members supporting moisture-permeable floors or roofs that are exposed to the weather shall be protected in accordance with CBC Section 2304.12.2.4.

**2.2.6** Ventilation of enclosed spaces beneath exterior balcony and elevated walking surfaces exposed to water shall comply with CBC Section 2304.12.2.5.

**2.2.7** CFS and structural steel members shall be protected by rust inhibitive coating (refer to CBC Section 2203A, AISI S220 Section A5 and AISI S240 Section A4).

### **2.2.8 Grade Clearance Requirements**

Framing protection/treatment and clearance requirements shall be provided in accordance with CBC Section 2304.12 and IR 16-1 Section 2.

**2.2.8.1** Under-floor clearance and the treatment of wood members in close proximity to exposed ground shall comply with IR 16-1 Section 2.2.1.

**2.2.8.2** Clearance from exterior grade to untreated wood construction shall comply with IR 16-1 Section 2.2.2.1.

#### **2.2.8.3 Exterior Grade Higher than the Bottom of Floor Framing**

Refer to IR 16-1 Section 2.2.2.2. Where the adjacent exterior grade is higher than the bottom of the floor framing on any side of the building, then all wood in the substructure and floor framing (including floor sheathing) shall be naturally durable or preservative-treated, all exposed steel and welds shall have a rust inhibitive coating, and the exterior wall envelope shall be weather-resistant with a continuous water-resistive barrier extending down to top of foundation wall. In addition, walls that retain earth and enclose interior spaces and floors below grade shall be waterproofed and dampproofed per CBC Section 1805A.1. Refer to Appendix A below for additional information. The following **exceptions** may apply:

**2.2.8.3.1** For single-story buildings with floor area not exceeding 2,160 Sq. Ft., the requirement for providing preservative-treated wood floor sheathing, wall framing, and wall sheathing need not be met except when the building uses light-framed walls with exposed wood structural panels as the lateral force resisting system. In addition, the protection requirements for exposed steel and welds as well as the requirements for providing a weather-resistant exterior wall envelope and continuous water-resistive barrier need not be met. This exception requires the flashing and mowstrip detail shown in IR 16-1 Figure 1 to be utilized and a request for a waiver of durability in accordance with IR 16-1 Section 2.3, resulting in a conditional approval. Refer to Appendix A below for additional information.

**2.2.8.3.2** For any building size, details providing an equivalent means of protection against decay, termite, and deterioration may be considered by DSA in lieu of providing naturally durable or preservative-treated wood per CBC Section 2304.12.1.2 and rust inhibitive coating over exposed steel and welds. This exception requires a form *DSA 1-AMM: Request for Alternate Design, Materials and Methods of Construction* to be submitted for DSA review and approval in accordance with IR 16-1 Section 2.2.2.2.

### **2.3 Concrete Curbs in Wood Framed Buildings**

CBC Section 2304.12.1.4.1 requires wood framed walls at shower or toilet rooms with more than two plumbing fixtures as well as walls at exterior locations to be constructed on concrete curbs at least 6 inches tall.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****2.3.1 Shower and Toilet Rooms**

In lieu of six-inch high concrete curbs, *IR 23-3: Concrete Curbs in Wood Framed Buildings* provides alternative means that will be accepted for stud wall and partition protection.

**Note:** If concrete curbs are omitted, the alternative means for protection shall be specified and detailed on the submittal for approval. Refer to Appendix A below for additional information.

**2.3.2 Exterior Walls**

Per IR 23-3, the concrete curb may be omitted if the exterior ground surface is paved for 18 inches and sloped away from the building, and the roof overhang length is equal to, or greater than, the height of the exterior wall.

For additional requirements and exceptions, such as mow strip and flashing details, refer to IR 16-1.

**2.4 Access Required for Under-Floor Utilities**

Refer to IR 16-1 for opening size requirement to all under-floor utilities such as electrical, mechanical, and plumbing.

**2.5 Floor Live Load and Roof Snow Load Postings**

Refer to IR 16-1 for signage requirements. Signs for load postings shall be posted in public view, whereas, Building Module Identification Labels do not necessarily have to be in public view.

**2.6 Utility and Service Lines**

Per ASCE 7 Section 13.6.9, all cables or flexible conduit across separation joints shall be designed to accommodate, without rupture or distress, differential movements from design displacements between cable connection points. PC drawings must indicate the maximum drift demand for each building option in the Design Information section of the coversheet. The DSA reviewer shall verify there is adequate flexibility provided in the utility and service lines, if applicable, at time of OTC or project submittal. Refer to Appendix A below for additional information.

**2.7 Canopies**

If attached canopies are part of PC, the load path to the building needs to be coordinated. The allowed locations of canopies need to be indicated. Partial plans must show any added roof framing or bracing required where canopies frame to the “long” side of a module. Wall panel design shall account for all loads imposed by the canopy.

**2.8 Column Schedules or Tables**

Where schedules or tables are used to define column size based on column height, clearly define in the structural drawings the basis for column height (i.e., column height measured from base plate to top of beam versus underside of beam). Tables need to consider different heights associated with sloped roofs (i.e., column size may need to change for differing heights due to sloped roofs).

**2.9 Ramps and Landings****2.9.1 Ramp/Landing Connection to Building**

A nominal connection between the ramp structure and relocatable building is permitted as required for non-structural purposes, but a standalone lateral system shall be provided for the ramp structure unless the following items are provided:

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- Provide calculations that demonstrate the connection between the ramp structure to building interface is capable of transferring the full seismic force from the ramp based on ASCE 7 Chapter 13 for “Egress Stairways not part of the building structure”, including any increase in force resulting from load eccentricities or irregular configurations. It shall also be demonstrated that the load path is complete, including collectors and chords as required to transfer forces to the building.
- Provide verification that the adjacent relocatable building has been designed to include any lateral forces imposed by the ramp system. Refer to Appendix A below for additional information.

**2.9.2 Ramp/Landing Foundation**

Refer to IR 16-1 for non-permanent foundations. The foundation shall be designed to prevent sliding on the supporting surface by attaching the wood foundation plates for the ramp to the ground with restraining devices, unless it can be demonstrated that the ramp can be supported by the adjacent building.

**2.9.3 Ramp/Landing Lateral System**

Refer to ASCE 7 Chapter 12 for lateral systems or Chapter 15 for “All other self-supporting structures...” utilizing  $R=1.25$ . Common systems include ordinary moment frames or ordinary concentrically braced frames. It is permitted to attach ramps to adjacent building for non-structural purposes, but a standalone lateral system for the ramp is still required unless it can be demonstrated that the ramp can be supported by the adjacent building per Section 2.9.1 above.

**2.9.4 Ramp/Landing Loading**

Design ramp for the more severe of 100 psf Live Load (unreducible) or applicable Snow Load where occurs.

**2.9.5 Ramp/Landing Floor Deck**

A positive connection between the decking and structure shall be provided for continuous load path.

**2.9.6 Ramp/Landing Posts**

For telescoping systems, a positive connection is required (e.g., bolts extending through both posts, sheet metal screw penetrating inner post, etc.) that is capable of transferring moment demands. Friction system (e.g., screw passing through outer post only) is prohibited for items supporting sustained gravity loads.

**2.9.7 Ramp/Landing Handrails**

Per CBC Section 1015, guards are required along the edge of ramp or landing if located more than 30" measured vertically to the grade below. If guardrail is required, the rail shall be designed to resist the greater of a 200-pound service load or 50 pounds per foot service load in accordance with CBC Section 1607A.8 and applied at 42 inches height per CBC Section 1015.3.

**2.9.8 Ramp/Landing Welding**

Per the Appendix of form DSA 103, welding inspection may be exempted by the design professional for relocatable ramps less than 30 inches above adjacent grade, except for base connections of cantilevered rail posts which do require welding inspection. If special inspection for structural welding for the ramp is exempted by the design professional on the form DSA 103, the following note shall be added to the PC drawings:



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“The design professional has exempted this ramp from special inspection requirements for material identification and structural welding. Ramp shall not be modified nor have shims added causing the distance between the highest ramp walking surface and the adjacent grade to be more than 30 inches. If this condition is not met, structural testing and/or inspection will be required to verify materials and structural welding. This applies to scopes of work including new construction, alteration, or relocation of the ramp.”

**2.10 Structural Tests and Special Inspection Requirements**

The tests mentioned are not inclusive of all inspections required for the project, but outline some of the unique or overlooked inspections that must be provided for a PC when applicable. Items apply to both in-plant and site inspections.

**2.10.1 Nondestructive Testing (NDT)**

NDT of complete joint penetration welds at moment-resisting connections shall comply with AISC 341 Chapter J per CBC Section 1705A.2.1. The example form DSA 103 on the PC drawings shall have the boxes checked for both Ultrasonic and Magnetic Particle Testing. In addition, the following note shall be provided on the PC drawings in the design parameters section to clarify the scope and frequency of testing – “The Nondestructive Testing Inspection is TBD by Architect of Record (AOR)/DSA per project specific requirements. Ultrasonic Testing (UT) shall be performed on 100 percent of the complete joint penetration (CJP) groove welds when the columns per Schedule on Sheets XX (xx to be filled out by PC applicant) have a thickness of 5/16" or greater. Magnetic particle testing shall be performed on 25 percent of all beam-to-column CJP groove welds.” Refer to Appendix A below for additional information.

NDT of complete joint penetration welds at gravity connections shall comply with AISC 360 Chapter N per CBC Section 1705A.2.1.

**2.10.2 Concrete Testing**

The example form DSA 103 on the PC drawings shall clearly identify each item that requires concrete material verification and testing such as concrete foundations and/or steel deck diaphragms with structural concrete fill where beams are designed as composite members or concrete fill is designed to transfer lateral loads.

**2.10.3 Post-Installed Anchors in Concrete**

Post-installed anchors in concrete shall be installed in accordance with CBC Section 1910A.5. Loads for testing shall be specified on the design documents.

**3. GRAVITY LOAD DESIGN****3.1 Roof Loading****3.1.1 Roof Dead Loads**

Per CBC Section 1606A.6, the design dead load shall provide for the weight of at least one additional roof covering in addition to other applicable loadings if the new roof is permitted to be applied over the original roofing without its removal.

**3.1.2 Partial Roof Live Loading**

Include the effects of partial live loading in the structural design as required per CBC Section 1607A.13.1 if it produces a more unfavorable load effect than the same intensity applied over the full member. In particular, this should be considered at balcony floor framing where occurs on two-story structures.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****3.1.3 Snow Load**

**3.1.3.1** The design information section shall state the snow and ice loads accounted for in the PC design. The PC drawings shall indicate 0 (zero) pounds per square foot (psf) if the design does not account for snow or ice loads.

**3.1.3.2** If the structure is designed for snow load, the design information section of the PC drawings shall include a note the same as or similar to the following: "Site application design professional and DSA plan reviewer shall verify the structure to be located at least xx feet from any adjacent higher structure" where the distance "xx" is calculated and stated by the PC applicant. Refer to ASCE 7 Section 7.7. If the horizontal separation from a higher structure is less than 20 feet and six times the vertical dimension separating the roofs, snow drift analysis shall be provided by the PC applicant, and the project is not eligible for OTC submittal.

**3.1.3.3** Refer to IR 16-1 Section 4.2 for roof snow load posting signage requirements. Signs shall be posted in public view.

**3.1.3.4** Effective seismic weight shall include snow load per ASCE 7 Section 12.7.2.

**3.1.4 Ponding Loads**

Roof configurations that allow for ponding are to be properly drained with primary and secondary drain systems, and roof framing design to account for any ponding buildup in event of primary blockage. Refer to ASCE 7 Chapter 8, and CBC Sections 1503 and 1611A.

**3.1.5 Areas of Roof Designated as Future Solar Zone**

Refer to PR 18-02, Energy Code Section 110.10, and CBC Sections 1603A.1.8.1 and 1607A.14.4.

For areas of the roof designated as a solar zone, the structural design loads for roof dead load and roof live load shall be clearly indicated on the construction documents, including any superimposed load for future solar components, etc. Include information to define the effective seismic weight considered in the structural design. Per Energy Code Section 110.10(b)4, the selection of the superimposed design loads for future solar components are solely at the discretion of the design professional and DSA will not mandate any additional prescribed minimum load.

Provide note on plans that future solar installation will require separate DSA application.

**3.1.6 Gypsum Board Soffits**

Gypsum board is acceptable for bracing of soffits, but not permitted for lateral resistance in shear walls per CBC 1617A.1.4. Ceiling detailing with gypsum board must be in accordance with *IR 25-3: Gypsum Board Ceiling Suspension Conventional Construction—One Layer*.

**3.1.7 Interior and Exterior Wall Framing**

Drawings shall show interior and exterior wall framing, clearly defining maximum openings and associated details. Details shall show connections that can resist concentrated reactions from jambs. Refer to Appendix A for additional information.

**3.1.8 Equipment Locations and Weights**

The roof plans shall show the allowed locations and weights of mechanical equipment, coordinated with the mechanical plans.

**3.1.9 Diagonal Bracing to Roof or Floor Framing**

When diagonal bracing (for ceilings, piping, etc.) normal to the purlin span direction is attached to the bottom flange or web of the framing member, justify framing and load path for transfer of

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loads to the diaphragm for the horizontal load, or provide blocking section between the two purlins. Design shall account for any horizontal loads imposed on purlin.

**3.2 Floor Loading****3.2.1 Partial Floor Live Loading**

Include effects of partial live loading in design of structure as required per CBC Section 1607A.13 if it produces a more unfavorable load effect than the same intensity applied over the full member. In particular, this should be considered at balcony floor framing where occurs on two-story structures.

**3.2.2 Wall Partition Parallel to Joists**

Check condition where partition wall extends parallel to floor joists. Provide additional joist under wall, add support blocking between joists, or provide calculations to check floor sheathing/deck for line load where wall above occurs.

**3.2.3 Decks Supporting Concrete Fill**

Include additional dead load due to calculated deck deflection between support members. In lieu of calculation or other substantiation (e.g., shoring during manufacturing to limit framing deflection during the pour), DSA will accept application of an additional 3 psf dead load for lightweight concrete fill or an additional 4 psf dead load for normal weight concrete fill. This additional dead load shall also be included in the effective seismic weight.

**3.2.4 Unbraced Ceiling Joists**

When horizontal loads are imposed on ceiling joists by ceiling bracing wires, mechanical, electrical, or plumbing components, fire sprinkler piping, etc., justify load path for transfer of loads to the floor or roof diaphragm. Provide solid blocking between the joists as needed to stabilize joists. Ceiling joists shall also be checked for vertical loading from compression struts unless the struts are detailed to extend vertically with positive attachment to floor or roof structure.

**3.2.5 Composite Action Between Upper and Lower Modules**

For multistory assemblies, the connection between the floor beam (upper level) and ceiling beam (lower level) shall be evaluated to consider composite action if these connections are detailed such that composite action could occur. Detailing to release composite action by providing slotted connections has been accepted by DSA as long as the connections can still transfer the uplift and shear forces from the upper to lower modules. Refer to Section 4.1.19 below for more information regarding upper to lower module connections.

**4. LATERAL LOAD DESIGN****4.1 Seismic (Requirements Applicable to All Systems)****4.1.1 Seismic Load Criteria**

The seismic design criteria upon which the PC design is based shall be stated in the design information section of the PC drawings in accordance with PR 07-01 and CBC Section 1603A.1.5.

**4.1.1.1 Maximum Seismic Force**

If the design is based upon the maximum  $S_s$  value for the state of California (ASCE 7-16 data), the PC can be used at any site in the state of California. Other  $S_s$  values are permitted but will limit the applicable site locations for the PC.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****4.1.1.2 Ground Motion Hazard Analysis**

The 2022 CBC adopts ASCE 7 with Supplement 3, which modifies Section 11.4.8. Due to the site-specific ground motion hazard analysis requirements of ASCE 7 Section 11.4.8, the seismic load criteria selected for the PC design per Section 4.1 above on Site Class D and E shall consider the Exceptions of ASCE Section 11.4.8, Items 1 and 2.

**4.1.1.3** The PC option for Site Class D shall include following note in the design information section: "Unless a site-specific ground motion hazard analysis is performed, the  $S_{M1}$  value increased by 50 percent shall be less than the design criteria stated herein."

**4.1.1.4** The PC option for Site Class E shall state in the design information section whether or not the PC design complies with the conditions of Exception 1 of ASCE 7 Section 11.4.8, Item 2.

**4.1.2 Maximum  $S_{DS}$  Value in Determination of  $C_s$  and E**

The base shear is permitted to be calculated using a cap on the maximum design spectral response acceleration parameter value of  $S_{DS}$  in accordance with ASCE 7 Section 12.8.1.3, provided that *all* of the noted criteria are met. The DSA reviewer shall verify any maximum limits applied to the base shear at time of OTC or project submittal. Refer to Appendix A below for additional information.

**4.1.2.1.1** The PC drawings shall demonstrate compliance with the required criteria (e.g., no irregularities, period less than 0.5 sec, rho equals 1.0, not Site Class E or F, RC I or II, etc.) in the Design Information section of the coversheet. For purposes of checking compliance with these criteria, the period shall be determined based on the actual properties of the structure, and not use the approximate period in ASCE 7 Section 12.8.2.1.

**4.1.2.1.2** The design information section of the PC drawings shall also contain a note stating the site-specific limitations of the design based on the  $S_{DS}$  cap and requiring these be verified by the site-specific project applicant: e.g., Site Class E not allowed, RC I or II, etc.

**4.1.2.1.3** Per PR 07-01 Appendix C, if a capped value of  $S_{DS}$  is used to determine  $C_s$ , the Design Information section of the coversheet shall list the  $S_{DS}$  (cap) used to determine  $C_s$  as well as the  $S_{DS}$  (no cap) used for verification of site-specific application and to determine other parameters such as non-structural component anchorage.

**4.1.3 Effective Seismic Weight****4.1.3.1 Snow Load**

If the site has a ground snow load greater than zero, effective seismic weight shall include snow load per ASCE 7 Section 12.7.2.

**4.1.3.2 Area of Roof Designated as Solar Zone**

Where portions of the roof are designated as solar zones, the building design loads due to future solar components shall be included in the effective seismic weight. For additional information, refer to Section 3.1.5 above.

**4.1.3.3 Partition Load**

If the PC drawings include an option for interior partition walls, the actual partition weight or a minimum weight of 10 psf of floor area shall be included in the effective seismic weight per ASCE 7 Section 12.7.2.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****4.1.4 Story Drift****4.1.4.1 Allowable Story Drift**

The allowable story drift limit shall be based on the limits for “All other structures” per ASCE 7 Table 12.12-1.

**Exception:** The less stringent drift limit listed in the first row of ASCE 7 Table 12.12-1 for “Structures, other than masonry shear wall structures, four stories or less above the base...” may be used if all interior walls, partitions, ceilings, and exterior wall systems have been designed and detailed to accommodate the story drifts unless specifically noted otherwise in this document.

**4.1.4.2 Story Drift Determination**

The design story drift shall be determined in accordance with ASCE 7 Section 12.8.6. For two-story buildings, the story drift shall be evaluated separately and shall not exceed the allowable story drift for each story height under consideration.

**4.1.5 Vertical Structural Irregularities**

For multistory assemblies, the soft story and weak story irregularities per ASCE 7 Section 12.3.2.2 need not be applied for modules in the stacked condition.

**4.1.6 Deformation Compatibility**

Buildings shall be designed and detailed for deformation compatibility as required per this section.

**4.1.7 Interior Non-bearing Non-shear Walls****4.1.7.1 In-plane Story Drift**

Interior non-bearing non-shear walls shall be designed and detailed to accommodate the building story drifts in accordance with ASCE 7 Equation 12.8-15. Special detailing is not required at wall intersections of conventional interior partitions to accommodate drift.

**Exception:** Design and detailing for story drift is permitted to be exempt for interior non-bearing non-shear walls in buildings satisfying drift limit for “All other structures” per ASCE 7 Table 12-12.1.

**4.1.7.2 Vertical Deflections**

Interior non-bearing non-shear walls shall be designed and detailed to accommodate vertical deflection in all cases.

**4.1.8 Exterior Non-bearing Non-shear Walls****4.1.8.1 In-plane Story Drift**

Exterior non-bearing non-shear walls shall be designed and detailed to accommodate story drift per ASCE 7 Section 13.5.3 for all cases except as noted below. Special detailing is not required at wall intersections or end conditions to accommodate drift.

**Exception:** Design and detailing for story drift is permitted to be exempt for exterior non-bearing non-shear walls in buildings satisfying drift limit for “All other structures” per ASCE 7 Table 12-12.1 for relatively light-weight exterior wall finishes that are attached to wall studs or substrate in a manner to accommodate cracking or spalling without becoming a falling hazard, including, but not limited to the following wall finishes:

- Wood siding (or like material such as Hardie-board) or metal siding.

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- Stucco: one or three-coat with lath properly fastened and embedded in the plaster coat in compliance with CBC Section 2512.1.
- Adhered veneer installed in accordance with CBC Chapter 14.
- Exterior insulation and finish system (EIFS) or other insulation board systems mechanically fastened to the studs or substrate and finished with similar light-weight finish materials.

**4.1.8.2 Vertical Deflections**

Exterior non-bearing non-shear walls shall be designed and detailed to accommodate vertical deflection.

**Exception:** For buildings satisfying drift limit for “All other structures” per ASCE 7 Table 12-12.1, exterior non-bearing non-shear walls are permitted to be exempt from accommodating vertical deflection, provided they are designed for their tributary dead, live and wind loads.

**4.1.9 Structural Separation**

All portions of the structure shall be designed and constructed to act as an integral unit in resisting seismic forces with interconnected modules unless each module is separated structurally by a distance sufficient to avoid damaging contact in accordance with ASCE 7 Section 12.12.3. PC drawings must indicate the maximum drift demand for each building option in the Design Information section of the coversheet. The DSA reviewer shall verify there is adequate structural separation, if applicable, at time of OTC or project submittal. Refer to Appendix A below for additional information.

**4.1.10 Interconnected Modules-Diaphragms, Chords, and Collectors**

**4.1.10.1** Buildings with interconnected modules and balconies shall comply with all applicable building code requirements, including, but not limited to the following items:

- Continuous load path and interconnection per ASCE 7 Section 12.1.3.
- Analysis of diaphragm flexibility per ASCE 7 Section 12.3.1, including torsional effects as applicable per ASCE 7 Section 12.8.4.
- Diaphragm design per ASCE 7 Section 12.10, including chord continuity and force transfer along and across the modlines and balconies.

**4.1.10.2** Buildings with interconnected modules shall specify and detail on the construction documents the maximum gap permitted between adjacent modules to ensure diaphragm and chord continuity in both directions. A positive connection such as through-bolts shall be designed and detailed to transfer tension forces between adjacent columns and beams on each side of the modline. Where the gap exceeds 1/8 inch, filler or shim plates shall also be added as required to effectively transfer compressive chord forces. In order to accommodate vertical movements between adjacent modules, the shim plates shall be detailed with a vertical slot or other mechanism to prevent vertical loads from being transferred across the joint.

**4.1.10.3** Buildings with interconnected modules and balconies shall be analyzed as an integral unit per ASCE 7 Section 12.12.3, by complying with the following provisions (i.e., “full building analysis” is required):

- Diaphragms shall be designed in accordance with ASCE 7 Section 12.10. Compliance may be deemed satisfied by demonstrating continuity of load path for transfer of chord and collector forces without necessarily requiring a single continuous tie element.

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- Diaphragms composed of structural concrete over metal deck shall be analyzed as rigid diaphragms. For levels with rigid diaphragms, the stiffness of the exterior wall framing supporting that level shall be included in the diaphragm flexibility and torsional analysis.
- Diaphragms composed of bare metal deck or wood sheathing shall meet the conditions noted in ASCE 7 Section 12.3.1 to confirm flexible diaphragm classification. This includes metal deck diaphragms topped with wood sheathing and/or non-structural concrete floor underlayment/insulating fill. The stiffness of the exterior wall framing supporting a flexible diaphragm level is permitted to be excluded from the diaphragm flexibility and torsional analysis.

**Exception:** For single-story buildings, interconnected modules may be exempt from analysis as an integral unit when *all* the following conditions are met:

- Each module has a Seismic Force-Resisting element (e.g., moment frame, shear wall, braced frame) on all four sides.
- All Seismic Force-Resisting elements in a direction are identical.
- The roof elevation is the same for all modules.
- The frames are not staggered (i.e., plan offset of modules is not allowed).

If any one of these conditions is not met, a “full building analysis” shall be provided in accordance with this section.

#### **4.1.11 Collector Design**

For exemption from load combinations with overstrength factor per ASCE 7 Section 12.10.2.1, Exception 1, the entire structure shall be light-frame construction, including the diaphragms, in addition to the requirements therein.

#### **4.1.12 Cold-Formed Steel Light Frame Flat-Strap Braced Roof Systems**

##### **4.1.12.1 General**

Where a diagonal flat-strap roof system is used as a means of lateral resistance, the shear resistance shall be determined based on the principles of mechanics. Sloping roofs shall consider and resolve any vertical load components. The requirements for strap-braced shear walls in AISI S400 Section E3 shall be applied to the roof structure as appropriate and as stated in this section.

**4.1.12.2** Connections shall be designed for the expected yield strength of the diagonal strap bracing member but need not exceed the amplified seismic load per AISI S400 Section E3.4.2 for light gauge steel (up to 3/16”) and AISC 341 Section F1.6 for heavier steel straps.

**4.1.12.3** Connections shall be welded unless the criteria in AISI S400 Section E3.4.1 are satisfied for light gauge straps.

**4.1.12.4** Per AISI S400 Section E3.4.2, collectors, connections of strap bracing, chords, boundary elements, and all other components and connections of the strap braced roof shall have required strength to resist loads from the expected strength of the strap braced wall but need not exceed the amplified seismic load with overstrength factor.

##### **4.1.12.5 Aspect Ratio**

Maximum total aspect ratio (length: width) for strapped diaphragm roof system shall be 4:1.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****4.1.12.6 Strap Configuration**

Per AISI S400 Section E3.4.1(c), provisions shall be made for pretensioning or other methods for installing tension-only strap bracing to guard against loose strap bracing. Since the strap bracing are expected to be installed taut, the slenderness ratio of the diagonal strap member may exceed 200.

**4.1.12.7** Per AISI S400 Section E3.4.2, the effect of eccentricity shall be considered in the design for connections, chord members, and collector elements.

**4.1.13 Attachment of Horizontal Plywood Diaphragms to CFS Framing Using Power Actuated Fasteners**

Refer to *BU 17-02: Power-driven Fasteners Attaching Wood Structural Panel Sheathing Diaphragms to Cold-formed Steel Framing*.

**Note:** DSA will accept the listed values in ET&F Fastening Systems, Inc. (ET&F's) International Association of Plumbing and Mechanical Officials (IAPMO) Report ER-335, subject to the requirements of IR A-5 (i.e., 0.8 multiplier for seismic loads) and 24 inches max CFS joist/rafter spacing as stated in bulletin *BU 20-02: Maximum Spacing of Cold-Formed Steel Joists for Diaphragms with Wood Structural Panels*.

**4.1.14 Maximum Spacing of CFS Joists for Diaphragms with Wood Structural Panels**

Per AISI S240 Section B5.4.1(k) and AISI S400 Section F2.4.1.1(k), the maximum spacing for CFS joists shall be 24 inches on center when the diaphragm is sheathed with wood structural panels.

**Note:** In prior code editions, it has been common practice for relocatable buildings to have CFS joists spaced up to 48 inches on center for diaphragms sheathed with wood structural panels. As stated in this section, the reference standards adopted by the 2022 CBC explicitly require a maximum CFS joist/rafter spacing of 24 inches on center which will be enforced for 2022 PC designs.

**4.1.15 Redundancy Factor**

A redundancy factor of 1.0 is permitted to be used for single-story or two-story buildings containing multiple modules designed as an integral unit that have a moment frame on all four sides of each module. For buildings with a single bay module (e.g., Toilet Building) or any other lateral system, a redundancy factor of 1.3 shall be used unless the conditions in ASCE 7 Section 12.3.4.2 are met.

**4.1.16 Overstrength Factor Reduction**

ASCE 7 Table 12.2-1 Footnote 'b' reduction to Omega for structures with flexible diaphragms is not applicable for light modular steel moment frame structures designed per CBC Section 2212A.2.

For all other structures, use of ASCE 7 Table 12.2-1 Footnote 'b' reduction to Omega is allowed only if flexible diaphragms are present at every level.

**4.1.17 Base Connection to Foundation**

The design shall explicitly state whether the base connection to the foundation is considered as pinned or fixed. The design and detailing shall match the fixity assumption.

**4.1.18 Design Load Combinations**

Verify basic seismic load combinations per CBC 1605A, where E is defined in ASCE 7 Section 12.4.2, incorporating vertical seismic load effects. Where seismic load effects including overstrength factor are required, the combinations of ASCE 7 Section 2.3.6 shall be applicable.



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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****4.1.19 Upper to Lower Module Connections**

The connection between upper and lower levels in two-story modules shall be designed to resist the maximum seismic load effect,  $E_m$ , per ASCE 7 Section 12.4.3. Provide calculations that demonstrate the connection is capable of transferring the seismic load between levels, including any increase in force resulting from load eccentricities (e.g., beam offsets, asymmetrical welds, irregular or offset anchor layout, etc.) in the connection.

**4.1.20 Cantilever and Offset Diaphragms, Including Clerestories**

**4.1.20.1** Design and detailing of cantilevered and/or vertically offset diaphragms shall satisfy the requirements of American Wood Council (AWC) Special Design Provisions for Wind & Seismic (SDPWS) Section 4.2. Provide calculations and details showing the values of loads to be transferred, as well as the load path, for offset diaphragms that are dependent on bending of vertical members to transfer loads back to main diaphragms.

**4.1.20.2** Vertical elements which span from high to low roof must satisfy continuous load path requirements of ASCE 7 Section 12.1.3 to keep the roofs moving in unison under lateral loads and satisfy deformation compatibility requirements per ASCE 7 Section 12.12.5. Deflection calculations shall consider out-of-phase vibration (i.e., check square root of the sum of the squares of roofs moving in opposite directions; other modal combination methods may also be used).

**4.1.20.3** If vertical members are utilized as collectors to transfer loads between diaphragms within the same story, then these vertical members and their connections must be designed using overstrength factor in accordance with ASCE 7 Section 12.10.2.1.

**4.2 Seismic (Light Modular Steel Moment Frame)****4.2.1 Maximum Design Dead Loads**

Refer to CBC Section 2212A.1.2 for 25 psf roof, 50 psf elevated floor, and 45 psf exterior wall for maximum dead loads and the following clarifications:

**4.2.1.1** Interior partitions need not be counted towards the dead load limits since partitions are classified as live load per ASCE 7 Section 4.3.2. However, partition load shall be included in effective seismic weight utilized for lateral analysis per ASCE 7 Section 12.7.2.

**4.2.1.2** Parapet weight need not be counted towards the roof dead load limit, but it shall be counted towards the exterior wall dead load limit and shall be included in effective seismic weight utilized for lateral analysis.

**4.2.1.3** When checking the dead load limit for elevated floors in two-story modules, the weight of both the "floor" of the upper module and the "ceiling" of the lower module shall be included. The "floor" and "ceiling" elements may be modeled separately in the engineering analysis for the building design, but for purposes of demonstrating compliance with CBC Section 2212A.1.2, the weights of those elements must be combined when checking the 50 psf limit.

**4.2.1.4** For areas of the roof designated as a solar zone on a solar ready building, any superimposed load for future solar components, etc., shall be included in the effective seismic weight. This weight shall also be counted towards the roof dead load limit for purposes of demonstrating compliance with CBC Section 2212A.1.2. Refer to Section 3.1.5 above for additional information.

**4.2.2 Beam-to-Column Strength Ratio**

Beam-to-column strength ratios shall be greater than or equal to 1.4 (i.e., weak-column strong-beam) in accordance with CBC Section 2212A.2.2.

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**Exception:** Beam-to-column strength ratios less than 1.4 are allowed if proven to be acceptable by analysis or testing, which shall be submitted to DSA using the Alternate Means and Methods procedure defined in California Administrative Code (CAC) Section 4-304 and CBC Section 104.11.

**4.2.3 Beam-to-Column Connection Design**

Refer to CBC Section 2212A.2.4 and the following clarifications.

**4.2.3.1 Beam-to-Corner Columns**

All connections shall be designed as moment-resisting connections.

**4.2.3.2 Moment Frame Connections**

**4.2.3.2.1** For frames with beam-to-column strength ratio greater than or equal to 1.4, connections shall have the design strength to resist the maximum seismic load effect,  $E_m$ , per ASCE 7 Section 12.4.3. Connections do not need to be designed for the expected yield strength of the connecting members.

**4.2.3.2.2** For frames using exceptions permitted in CBC Section 2212A.2.2 with beam-to-column strength ratio less than 1.4, connections shall be designed for a required flexural strength that is equal to the expected beam flexural strength using the expected yield stress per AISC 341 Section E1.6b.

**4.2.4 Connection Between Levels**

The connection between upper and lower levels in two-story modules shall be designed to resist the maximum seismic load effect,  $E_m$ , per ASCE 7 Section 12.4.3.

**4.2.5 HSS Connections**

For Hollow Structural Sections (HSS) beam to HSS column connections at the roof or floor level, the connection design is permitted to be based on AISC 360, Chapter K.

**4.2.6 Welding**

Refer to CBC Section 2212A.2.3.

**4.2.7 Moment Frame Beams with Web Openings**

**4.2.7.1** Web openings shall not be permitted in moment frame beams over a length equal to the beam depth away from the face of column. All openings shall be clearly located on plan (preferably near midspan of beam where shear demand is typically lowest) with opening size and height relative to top flange shown.

**4.2.7.2** The design shear force used to evaluate the beam opening shall be based on the maximum probable moment ( $M_{pr}$ ) that can be developed (i.e.,  $2M_{pr}/L$ ) or load combinations with overstrength factor, whichever is less. The steel reinforcement around the opening shall be sized to develop the full yielding of the effective section to achieve the required bending or shear strength.

For additional requirements, refer to the design references noted below:

**4.2.7.3 Structural Steel**

Refer to AISC Design Guide 2 – Design of Steel and Composite Beams with Web Openings, for design of openings.

**4.2.7.4 Cold-Formed Steel**

Per AISI S240 Section C2.1, holes in webs of CFS framing members shall be in conformance with an approved design based on AISI S100 such as Cold-Formed Steel Engineers Institute

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(CFSEI) Tech Note G900-15. Webs with holes not conforming to AISI S100 shall be reinforced or patched in accordance with a consensus design standard.

**4.3 Seismic (Flat Strap Braced Shear Walls)****4.3.1 General**

**4.3.1.1** Where diagonal strap bracing is provided for lateral resistance, the connections shall be designed for the expected yield strength of the diagonal strap bracing member but need not exceed the amplified seismic load per AISI S400 Section E3.4.2 for light gauge steel (up to 3/16") and AISC 341 Section F1.6 for heavier steel straps.

**4.3.1.2** Connections shall be welded unless the criteria in AISI S400 Section E3.4.1 are satisfied for light gauge straps.

**4.3.1.3** Per AISI S400 Section E3.4.2, collectors, connections of strap bracing, chord studs, vertical boundary elements and anchorage connected thereto, and all other components and connections of the strap braced wall shall have required strength to resist loads from the expected strength of the strap braced wall but need not exceed the amplified seismic load with overstrength factor.

**4.3.2 Vertical Boundary Members and Anchorage**

Per AISI S400 Section E3.4.1(d), anchorage of vertical boundary members shall be provided such that the bottom track does not resist uplift by bending of the track web.

**4.3.3 Aspect Ratio**

Per AISI S400 Section E3.4.1(b), light gauge flat strap braced wall shall have an aspect ratio less than 1.9:1 unless a lateral frame analysis is performed based on the assumption of full joint fixity.

**4.3.4 Strap Configuration**

**4.3.4.1** Per AISI S400 Section E3.4.1(c), provisions shall be made for pretensioning or other methods for installing tension-only strap bracing to guard against loose strap bracing. Since the strap bracing are expected to be installed taut, the slenderness ratio of the diagonal strap member may exceed 200.

**4.3.4.2** Per AISI S400 Section E3.4.2, the effect of eccentricity shall be considered in the design for connections, chord studs, holdowns and anchorage.

**4.4 Seismic (Light Framed Wood Wall with Wood Structural Panels Rated for Shear Resistance)****4.4.1 General**

**4.4.1.1** Per the American Wood Council (AWC) Special Design Provisions for Wind & Seismic (SDPWS), Section 4.3.4, maximum aspect ratio for blocked shear walls is 3.5 to 1, with strength reductions from National Design Specification (NDS), Section 4.3 applied. Full design values can be used for aspect ratios up to 2 to 1.

**4.4.1.2** Unblocked shear walls are not permitted per CBC Section 2301.1.4.

**4.4.2 Holdowns****4.4.2.1 Holdown Load Capacities**

In accordance with IR A-5, design capacity values shall be 80 percent of the listed seismic load capacity for manufactured wood construction connectors unless the values listed in the evaluation report were established on the basis of cyclic test results.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****4.4.2.2 Holdown Body and Anchor Bolt Area**

Per CBC Section 1617A.1.16 Exception 1, the wood-framed shear wall holdown body and anchor bolt area are exempt from amplified seismic load with overstrength factor.

**4.4.3 In-Plane Shear Anchorage for Shear Walls**

Foundation anchor bolts or screws with a valid evaluation report for this application shall have plate washers meeting the requirements of the AWC SDPWS Section 4.3.6.4.3.

**Note:** The exception in AWC SDPWS Section 4.3.6.4.3 allowing cut washers is not permitted per CBC Section 2301.1.4 Item 3.

**4.4.4 Shear Wall Construction**

Structural sheathing shall be applied directly to the framing per CBC Section 2301.1.4 Item 6. For other system requirements, refer to AWC SDPWS Section 4.3.7.1.

**4.5 Wind Design****4.5.1 Metal Roof Panel Systems**

Metal roof panel systems shall comply with CBC Sections 1504.4 and 1507.4. A manufacturer, product, and basis of approval for the metal roof panel system and fastener parts/layout shall be specified. The interpretations noted below do not preclude the project from meeting the other requirements of CBC Chapter 15 (e.g., fire classification, insulation, etc.).

**4.5.2** The PC drawings shall fully detail the panel clip spacing and fasteners, metal material specification, panel profile, thickness, etc.

**4.5.3** The underlayment, flashing and other waterproofing shall be fully detailed on the PC drawings and shall comply with CBC Chapter 15.

**4.5.4** The wind load uplift resistance provided by the panels and their connections shall be based on test data in accordance with ASTM E1592, Underwriters Laboratory (UL) 580, UL 1897, or Factory Mutual (FM) 4474 as appropriate by an independent accredited laboratory. Qualification of the metal roof panel system shall be provided in accordance with the one of the following methods:

**4.5.4.1** If the system has a UL or FM certification or has an evaluation report issued by an accepted agency per IR A-5, this basis of approval shall be denoted on the PC drawings. A copy of the UL or FM product report, if applicable, shall be submitted to substantiate the design capacity of the products.

**4.5.4.2** If the system does not have a qualified certification per Section 4.5.4.1 above, a wind uplift test report by an independent accredited testing laboratory shall be submitted with the PC project. The test results shall be interpreted for applicability and adjusted for factor of safety in accordance with AISI S100 Section I6.3.1 by a California registered Structural Engineer. The testing laboratory shall be accredited by International Organization for Standardization (ISO) 17025 in accordance with International Code Council Evaluation Service (ICC-ES) AC85. Other accreditations (e.g., ISO 17020) may be acceptable with DSA approval.

**4.5.5 Wind Loads on Trusses, Girders, Beams and Purlins**

For conditions where trusses, girders, beams, or purlins are subject to compression or wind uplift, the bottom chord/flange shall be braced or justified by calculation as not requiring bracing where reverse curvature occurs or where top of exterior stud wall imparts a horizontal reaction to bottom chord/flange. In addition, member end connections shall be checked for wind uplift where occurs.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****4.5.6 Parapet Vertical Support Post Layout**

Parapet vertical support post locations shall be shown on a typical layout plan. Indicate where posts occur on the “long” and “short” sides of a module with details showing how base moments from the parapet supports are transferred into the supporting structure.

**4.5.7 Exterior Wall Insets**

Exterior non-bearing non-shear walls are permitted to be “inset” if details are provided that clearly define acceptable wall locations and demonstrate how out-of-plane wind loads are transferred to the roof or floor structure. A slip track is required at the top of wall to allow for both in-plane and vertical movement. The floor system shall be checked per Section 3.2.2 above if the inset wall extends parallel to floor joists.

**4.5.8 Rooftop Photovoltaic Panels**

Rooftop PV panels shall be designed in accordance with DSA IR 16-8

**5. FOUNDATION****5.1 General**

Refer to CBC Section 1808A.1.

**5.2 Allowable Soil Pressure and Bearing**

Maximum soil pressure and bearing values shall be in accordance with Class 5 soil as specified in CBC Table 1806A.2 unless justified by a site-specific geotechnical report. Alternative values may be included as an option if the PC drawings clearly indicate in the Design Parameter Information section on the PC drawings that a site-specific geotechnical report is required at the time of site application. Refer to Appendix A below for additional information.

**5.2.1** An allowable stress increase is not permitted for Basic Allowable Load Combinations, including footings, per ASCE 7 Section 2.4. An allowable stress increase is permitted for Alternative Basic Load Combinations per CBC Section 1605A.2.

**5.2.2** Reduction of foundation overturning per ASCE 7 Section 12.13.4 is permitted with Basic Allowable Load Combinations but not permitted for Alternative Basic Load Combinations per CBC Section 1605A.2

**5.3 Liquefiable Soil or Site Class F**

A PC’s option shall not include liquefiable soil nor site Class F. If the structure is located in an area with liquefiable soil or Site Class F, OTC review is not allowed, and site-specific project submittal is required. If the site is not in a mapped liquefaction hazard zone, it may be presumed that no liquefaction hazard exists on that site unless a site-specific geotechnical report identifies such hazard.

**5.4 Foundations on or Adjacent to Slopes**

PC drawing shall specify minimum setback limits (values are required) of the structure per CBC Section 1808A.7 for building clearance, foundation setback, etc. for protection from slope drainage, erosion, and shallow failures. If setback limits are smaller than CBC requires, site-specific geotechnical report is required. Refer to Appendix A below for additional information.

**5.5 Concrete Mix**

In addition to those requirements dictated by the PC design, the concrete mix used in the foundation elements shall comply with the durability requirements of American Concrete Institute (ACI) 318 Section 19.3. The PC drawings shall account for the dependency of these durability requirements on site-specific characteristics.

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**5.5.1** When the PC drawings do not require a site-specific geotechnical report that quantifies sulfate content in the soil, the PC drawings shall require a concrete mix shall complying with one of the following per ACI 318 Table 19.3.2.1.

**5.5.1.1** Maximum water/cement ratio of 0.45; minimum compressive strength of 4,500 pounds per square inch (psi); Type V cement plus pozzolan or slag cement complying with Footnote 7; and prohibition of admixtures containing calcium chloride.

**5.5.1.2** Maximum water/cement ratio of 0.40; minimum compressive strength of 5,000 psi; Type V cement complying with Footnote 8; and prohibition of admixtures containing calcium chloride.

**5.5.2** When the PC drawings require a site-specific geotechnical report that quantifies sulfate content in the soil, the PC drawings shall clearly state the exposure class for each category (i.e., F, S, W and C) or combination thereof the PC design is approved for. The maximum water/cement ratio, minimum compressive strength, cementitious material requirements, and admixture limitations shall be stated on the PC drawings for each approved case.

**5.5.3** Both approaches given in Sections 5.5.1 and 5.5.2 above can be included on the PC drawings as alternate options in accordance with Section 1.4 above.

**5.5.4** Concrete exposed to thaw and freeze cycles shall be air entrained per ACI 318 Section 19.3.3.1.

## **5.6 Foundation Design Load**

The design of the foundation and connections shall include overstrength factor in accordance with CBC Section 1617A.1.15.

## **5.7 Permanent Foundations**

**5.7.1 General:** Refer to CBC Section 1808A.1.

### **5.7.2 Lateral Force Distribution to Concrete Piers**

Lateral forces shall be distributed to each modline foundation element on a tributary area basis. Along interior modlines, the lateral force shall be distributed to each foundation element (i.e., interior piers, partial strip footing) based on relative rigidity, or the tributary load may be dragged to the end strip footings on that modline.

## **5.8 Foundations Supporting Raised Floors**

**5.8.1** Concrete piers supporting raised floor framing shall be evaluated for overturning stability.

**5.8.2** Per ASCE 7 Section 12.1.4, positive attachment of raised floor framing to interior pier supports is required.

## **5.9 Anchorage and Shear Connections to Concrete**

**5.9.1** The connection of the superstructure to foundation shall be designed for forces per CBC Section 1617A.1.16 and ACI 318 Chapter 17. "Can-outs" (i.e., temporary blockout in concrete to allow anchor bolt installation after concrete placement) are not permitted.

**5.9.2** Determination of shear strength of sill bolts for walls in light-frame structures may be eligible for exemption from portions of ACI 318 Chapter 17 requirements in accordance with CBC Section 1905A.1.8.

**5.9.3** Post-installed anchor placement must comply with ACI 318 Chapter 17 and product evaluation report per IR A-5. Post-installed anchors shall be tested per CBC 1910A.5 and test loads for post-installed anchors shall be shown on the construction documents in accordance with CBC Section 1910A.5.1.

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**5.9.4** Connections to the foundation are required on all four sides of each module. These connections may be achieved through a combination of primary shear connections along with nominal anchorages. Nominal anchorages to the supporting foundation shall be spaced as needed to ensure structural integrity of all portions of the structure. Floor beams shall be designed to span vertically between points of anchorage.

**Exception:** If it can be shown by engineering analysis that a load path meeting the requirements of Section 5.9.5 below is provided without anchoring all four sides of a module and that the structural integrity of all parts of the structure is maintained, then the requirements in Section 5.9.4 above need not be met. The analysis shall demonstrate the integrity of any unanchored foundation walls with seismic and soil loads and shall show that the horizontal deflection of floor elements, diaphragms, and foundation elements will not cause the floor beams to lose vertical support at any point along the beams.

**5.9.5** All elements of the connection to the foundation, including anchor bolts, and the entire load path to the connection shall be analyzed for the effects of load eccentricities, prying action, stiffness compatibility, load reversals and appropriate boundary conditions to ensure all code requirements are satisfied.

**5.9.6** The connection shall also be designed for uplift where occurs based on the maximum seismic load effect,  $E_m$ , per ASCE 7 Section 12.4.3. Provide calculations that demonstrate the connection is capable of transferring the seismic load into the foundation, including any increase in force resulting from load eccentricities (e.g., member offsets, asymmetrical welds, irregular or offset anchor layout, etc.) in the connection. In order to minimize the amount of load eccentricity in the connection between perimeter floor framing and foundation anchor plates, it is recommended to provide welds connecting both sides of the framing to the embed plate and aligning the beam centroid over the anchor to the greatest extent possible.

**5.9.7** Per CBC Section 1617A.1.20, power actuated fasteners (PAF) shall not be allowed for anchorage into concrete for exterior wall applications. PAF are permitted for anchorage to steel on exterior wall applications in accordance with an approved evaluation report.

### **5.9.8 Wood Sill Plates**

Wood sill plates with anchorage to concrete under exterior walls, bearing walls and shear walls shall be bolted to concrete and properly spaced with clearance to end of sill plate in accordance with CBC Section 2304.3.4 Item 2.

### **5.10 Non-Permanent Foundations**

Refer to CBC Section 1808A.1 and IR 16-1.

#### **5.10.1 Maximum Building Area Permitted**

Refer to IR 16-1 Section 3.2.2 for maximum building area permitted for relocatable buildings made up of one or more structurally separate portions and supported by wood foundations.

#### **5.10.2 Shared Wood Foundation Along Modlines**

Design foundation along modlines for the tributary width of both modules.

#### **5.10.3 Wood Foundation Splices**

Verify splices at wood sill plates are adequate to transfer horizontal reactions to sill restraint.

## **6. ACCESSIBILITY**

### **6.1 PC Submittals**

Refer to PR 07-01 Section 4.3.

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Accessibility requirements shall comply with CBC Chapter 11B. Appropriate details shall be provided on plans to assure compliance with all applicable code requirements.

**7. FIRE AND LIFE SAFETY REQUIREMENTS****7.1 PC Submittals**

Refer to PR 07-01 Section 4.2.

**7.2 Type of Construction**

Specify type of construction per CBC Chapter 6.

**7.3 Hazardous Fire Area**

For relocatable buildings intended for installation within designated hazardous fire areas, construction and materials shall also comply with CBC Chapter 7A. Plans shall include a notation indicating that the relocatable has been designed per CBC Chapter 7A compliance. Additionally, the plans shall indicate the specific construction compliance method for the building features as outlined in Sections 707A.3 through 707A.10, 708A, and 709A, as applicable.

**7.4 Total and Allowable Areas of Structure**

Specify total area of structure and provide calculations indicating that total area is less than allowable area as determined per CBC Section 506.2 based on type of construction, proposed occupancy classification(s), and that any intended area increases based on frontage are detailed on the plans and comply with CBC Section 506.3.

**7.5 Use and Occupancy Classification(s)**

Specify proposed use and occupancy classification(s) per CBC Chapter 3.

**7.6 Occupant Load**

Specify proposed occupant load based on function per CBC Table 1004.5. The following may be required where occupant load exceeds 49.

**7.6.1** Two or more exits per CBC Table 1006.2.1 and Table 1006.3.3. Exit and exit access doorway configuration shall comply with CBC Section 1007.

**7.6.2** Panic hardware per CBC Section 1010.2.9.

**7.6.3** Emergency power for egress illumination (including exterior landings) per CBC Section 1008.3.

**7.6.4** Exit signs with backup power per CBC Section 1013.

**7.6.5** Occupant load sign per CBC Section 1004.9.

**7.7 Roof Fire Hazard Classification**

Specify roof fire hazard classification per CBC Section 1505. For relocatable buildings located in a designated hazardous fire area, roof assemblies shall also comply with CBC Chapter 7A.

**7.8 Fire Extinguishers**

Provide fire extinguisher locations and specify type per California Fire Code (CFC) Section 906.

**7.9 Interior Finishes**

Interior wall, ceiling, floor, and decorative finishes shall comply with CBC Chapter 8, and CCR Title 19.



**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****7.10 Group E Door Hardware**

Group E doors may be required to be lockable from the inside per CBC Section 1010.2.8.2.

**7.11 Stairways**

Stairways shall comply with CBC Section 1011.

**7.12 Ramps**

Ramps shall comply with CBC Section 1012.

**7.13 Handrails**

Handrails shall comply with CBC Section 1014.

**7.14 Guards**

Guards shall comply with CBC Section 1015.

**7.15 Safety Glazing**

Safety glazing is required in hazardous locations (in doors, adjacent to doors, in windows, etc.) per CBC Section 2406.4.

**7.16 For Multistory Relocatable Buildings**

Elevator design and construction shall comply with the following:

**7.16.1** CBC Chapter 30.

**7.16.2** CFC Section 606.

**7.16.3** California Code of Regulations, Title 8, Division 1, Chapter 4, Subchapter 6, Elevator Safety Orders.

**8. SUSTAINABILITY REQUIREMENTS**

PC designs for relocatable buildings must comply with the mandatory measures of the California Green Building Standards Code (CALGreen) and the California Energy Code (Energy Code).

**8.1** Plans must show the primary exterior entries are protected from water intrusion by adding a recessed door, awning, or roof overhang at least 4 feet in depth in addition to using nonabsorbent floor and wall finishes within 2 feet around and perpendicular to such openings in accordance with CALGreen Section 5.407.2.2.

**8.2** For a more detailed list of CALGreen/Energy items that are required for all PC submittals, refer to IR 16-1, PR 07-01, *PR 18-02: Pre-Check (PC) Permanent Modular or Relocatable Building Designs CALGreen/Energy Code Compliance Review*, form *DSA 403-PC: CALGreen and Energy Code-Compliance Checklist for Pre-Checked (PC) Permanent and Modular Relocatable Building Designs*, and *IR N-3: PC Energy Code Requirements for Photovoltaic and Battery Systems*.

**8.3 Required Photovoltaic (PV) Systems Energy Review**

When a PV System is required per the California Energy Code for a PC configuration, the system power requirements shall be clearly delineated on the PC plans in the Design Information section for the PC. See Section 1.7 above for design and submittal requirements of the PV system.

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### REFERENCES:

2022 California Code of Regulations (CCR) Title 24

Part 1: California Administrative Code (CAC), Section 4-304.

Part 2: California Building Code (CBC), Chapters 3, 6, 7A, 8, 10, 11B, 14, 15, 16A, and Sections 104.11, 107.2.5, 110.3.6, 202, 506, 1202, 1705A.2.1, 1804A.4, 1806A.2, 1808A, 1910A.5, 2203A, 2211A, 2212A, 2301.1.4, 2304.3.4, 2304.12, 2406.4, 2512.1.

Part 9: California Fire Code (CFC), Section 906.

Part 11: California Green Building Standards Code (CALGreen), Section 5.407.2.2.

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This IR is intended for use by DSA staff and by design professionals to promote statewide consistency for review and approval of plans and specifications as well as construction oversight of projects within the jurisdiction of DSA, which includes State of California public schools (K–12), community colleges and state-owned or state-leased essential services buildings. This IR indicates an acceptable method for achieving compliance with applicable codes and regulations, although other methods proposed by design professionals may be considered by DSA.

This IR is subject to revision at any time. Please check DSA's website for currently effective IRs. Only IRs listed on the webpage at <https://www.dgs.ca.gov/dsa/publications> at the time of project application submittal to DSA are considered applicable.

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**PRE-CHECK (PC) DESIGN CRITERIA FOR RELOCATABLE BUILDINGS: 2022 CBC****APPENDIX A: SITE-SPECIFIC APPLICATION GUIDE**

The following notes are provided as a guide to assist design professionals and DSA plan reviewers when preparing and reviewing site-specific project applications that incorporate PC relocatable buildings designed in accordance with this IR. This appendix is not intended to be an all-inclusive list of design and submittal requirements but rather is an aid to identify aspects of the design criteria described in this IR of particular interest to its site application.

- Verify site-specific suitability of the PC Relocatable Building including all parameters in PL 07-02 Section 3.
- Verify site-specific requirements of PL 07-02 Section 4 are met.
- Verify the RC and occupancy classification of the site-specific design is compliant with the Design Information section of the approved PC. RC is determined by the requirements of CBC Section 1604A.5. As described in CBC Table 1604A.5, this determination is based on the nature of the Occupancy and Occupant Load. Refer to Sections 1.8 and 7 above for additional information.
- Where structures have two or more portions that are structurally separated and each have separate means of egress, the RC of each portion shall be separately determined in accordance with CBC Section 1604A.5.1.
- Because Occupant Load is a Fire and Life Safety issue, buildings or structures that are structurally separate but share the same code-required means of egress shall be considered together for Occupant Load and, therefore, RC determination.
- Where a structurally separate portion is in the path of egress for other structurally separate portions, the Occupant Load used to determine the RC of the first portion shall be as defined in CBC Section 202 and shall be computed in accordance with CBC Section 1004.
- The Occupant Load for any structurally separate portion shall be the sum of the Occupant Load originating in that portion plus the number of occupants egressing through it from other structurally separate portions.
- The number of occupants egressing through a structurally separate portion shall be based on an exiting analysis that satisfies the requirements of CBC Chapter 10 for Means of Egress as defined in CBC Section 202, including consideration of the requirements of CBC Section 1029 for Assembly Occupancies where applicable.
- Review the appendix of the site-specific form DSA 103 for any exemptions from the required structural tests and special inspections. Applicability and consideration of exemptions may be discussed during plan review for site-specific applications and shall be justified by the applicable project design professional for DSA review and approval. Refer to Section 1.3 and 2.10 above for additional information.
- In addition to the requirements of PL 07-02 Section 4.9, if the site is located in a flood zone other than Zone X, verify a validation letter from a geotechnical engineer is provided. Refer to Section 1.9 above for additional information.
- Geohazard Reports:
  - If the site-specific building design exceeds 4,000 Sq. Ft. in plan area or is located within state or local geologic hazard zones, verify submittal and approval of a geohazard report by CGS in accordance with IR A-4. The structures may be split into multiple seismically separated structures to stay below the 4,000 Sq. Ft. trigger. Refer to Section 1.10 above for additional information.

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- For single-story relocatable buildings, located within state or local geologic hazard zones, that are less than 2,160 Sq. Ft. on non-permanent foundations, verify a geohazard report is provided in accordance with IR A-4. DSA may waive the requirement for submittal and approval of the geohazard report by CGS if the report indicates there are no geologic hazards at the site. Refer to Section 1.10.2 above for additional information.
- If the site-specific building is placed on raised wood foundations, sloped drainage is not required away from the building, but verify there is an adequate slope at the site to prevent water from ponding beneath and immediately adjacent to buildings. For other conditions, such as lowered basin under building or concrete foundations, verify there is sloped drainage away from the building in accordance with CBC Section 1804A.4. Refer to Section 2.2.2 above for additional information.
- Verify spot elevations are provided on the site plan to ensure the following criteria are met:
  - Proper clearance shall be maintained from grade to untreated wood construction per IR 16-1 Section 2. Refer to Section 2.2.8. above for additional information.
  - For permanent foundations, verify the foundation height does not exceed the PC design limits. Refer to Section 5.8 above for additional information.
  - For non-permanent foundations, verify the distance between the underside of the lowest floor framing member and the top of grade under the building does not exceed 18 inches. Refer to IR 16-1 Section 3.2 above for additional information.
- Verify under-floor ventilation is provided in accordance with CBC Section 1202.4. Where the adjacent exterior grade is higher than the bottom of the floor framing on any side of the building, verify the following criteria are met. Refer to Sections 2.2.3 and 2.2.8 above for additional information.
  - Verify all wood in the substructure and floor framing (including floor sheathing) is naturally durable or preservative-treated, all exposed steel and welds have a rust inhibitive coating, and the exterior wall envelope is weather-resistant with a continuous water-resistive barrier extending down to top of foundation wall in accordance with IR 16-1 Section 2.2.2.2.
  - For single-story buildings with floor area not exceeding 2,160 Sq. Ft. that elect for a conditional approval with substandard foundation materials or substandard decay and termite prevention detailing, verify that flashing, mowstrip and concrete cover are provided in accordance with IR 16-1 Figure 1. In addition, verify a written request for a waiver of durability has been submitted to DSA in accordance with IR 16-1 Section 2.3.
  - For permanent foundations, verify details of vent wells are shown on the site plan as required to provide cross ventilation of the under-floor space.
  - For non-permanent foundations, verify there is an acceptable method to provide cross ventilation of the under-floor space.
- If the site-specific building design includes shower or toilet rooms with more than two plumbing fixtures, verify curbs or an alternate means of protection is provided in accordance with CBC Section 2304.12.1.4.1 and IR 23-3. If the PC drawings do not provide details for this condition, they shall be provided on the site-specific drawings. Refer to Section 2.3.1 above for additional information.

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- If the site has a ground snow load greater than zero, verify the relocatable building is positioned with sufficient distance from any adjacent structure as defined on the PC drawings. If the horizontal separation is less than 20 feet, snow drift analysis shall be provided by the PC applicant, and the project is not eligible for OTC review. Refer to Section 3.1.3 above for additional information.
- Verify utility and services lines crossing building separation joints are designed to accommodate, without rupture or distress, differential building movements as defined on the PC drawings. Refer to Section 2.6 above for additional information.
- Verify the site-specific building design of window and/or door openings falls within the dimensional limits and wall framing conditions covered on the PC drawings. Refer to Section 3.1.7 above for additional information.
- If soil pressure and bearing values exceed Class 5 soil as specified in CBC Table 1806A.2, a site-specific geotechnical report shall be provided at the time of site application to justify values used. Refer to Section 5.2 above for additional information.
- If the building is placed adjacent to a slope, verify the building location complies with the setback requirements defined on the PC drawings. Refer to Section 5.4 above for additional information.
- If the building is placed on non-permanent foundations, verify the relocatable building is a single-story structure with a floor plan area that does not exceed 2,160 Sq. Ft. The floor area in consideration shall include the combined area of any adjacent buildings if interconnected for use by doors or passage ways, even if the adjacent buildings are structurally separated. Refer to Section 5.10.1 above for additional information.
- If the site-specific building is designed for Site Class D or E, verify if a site-specific ground motion hazard analysis is required. Refer to Section 4.1.1.2 above for additional information.
- If a ground motion cap is applied to the value of  $S_{DS}$  in determination of the seismic base shear, verify the required criteria per ASCE 7 Section 12.8.1.3 are met at time of OTC or project submittal. In addition, verify the value of  $S_{DS}$  for the site-specific application does not exceed the value of  $S_{DS}$  (no cap) listed on the PC drawings. Refer to Section 4.1.2 above for additional information.
- Verify the building location on the site complies with the dimensional requirements for separation from existing buildings or other new buildings as defined on the PC drawings. Unless a detailed analysis is provided, the movement of an adjacent existing building shall be assumed to be that corresponding to the maximum drift allowed by the governing code at the time of the existing building's design or construction. Refer to Section 4.1.9 above for additional information.
- Verify the scope of nondestructive testing of complete joint penetration welds listed on the site-specific DSA 103 is in accordance with the example DSA 103 on the PC drawings and the design option(s) utilized in the site-specific relocatable building design. Refer to Section 2.10 above for additional information.
- If the building has a ramp, verify the ramp structure and configuration matches the ramp included in the approved PC documents or has a separate PC approval that is appropriate for the site-specific project. Otherwise, the project is not eligible for OTC review, and additional items must be provided for review in accordance with Section 2.9.